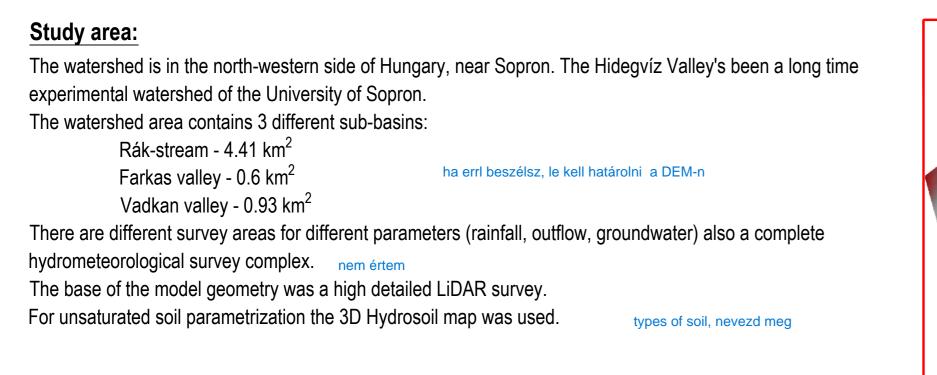
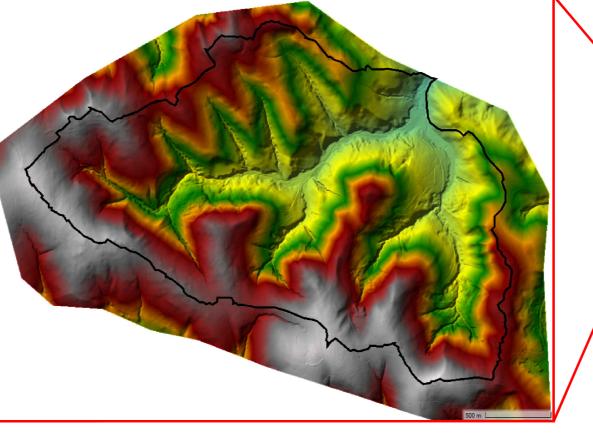
# Impact of hydrological, hydraulic modelling approach to a flash flood event in the Hidegvíz watershed in Hungary

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# SZÉCHENYI István SZE-GYŐR UNIVERSITY

szerintem ez túl nagy





Measured Data (provided by the university of Sopron Rainfall date Runoff at station Date

#### **Boundary conditions:**

In this study the data provided by the researchers of the University of Sopron were used as boundary conditions and measured data for calibration on both models.

Upper boundary condition:

Precipitation data from the hydrometeorological station, for the rainfall event on 06/07/2012 14:30 - 16:50

#### Outflow time series for calibration:

Calculated flow time series from the cauging station at the outflow area on 06/07/2012 14:30 -07/07/2012 14:30



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Hydrometeorological station and gauging weir Measured cumulative rainfall time series Outflow time series from measured data

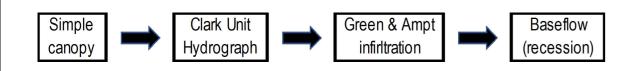
**Models** 

#### jelmagyarázat nagyobb nem látszik

DEM of the Hidevíz Valley

#### Hydrological flood modeling:

Watershed model with lumped parameterization on each sub-basin (HMS). The used modules in the model was built up by the schema below:





Hydrological model geometry

#### **Calibrated values:**

táblázat, simple caroby [mm] = 5 tc [h] = 1.68, 2.02, 2.68 tR [h] = 0.9, 0.9, 1.2 K [mm/h] = 27.596 Sf [mm] = 216.55 Initial deficit = 0.173 Imperviousness [%] = 0.01 Initial flow [cms] = 0.0014, 0.0009, 0.0065 nem kell Recession constant = 0.2Ratio to peak = 0.32, 0.32, 0.09

#### Hydrodynamical flood modeling:

2D FVM surface flow model, with solver based on SWE (shallow water equations) (HEC-RAS). In this study a new solver type was used, SWE-LIA (local inertia):

$$\frac{\partial V}{\partial t} + \frac{(V \cdot \nabla)V}{f_c k} + f_c k \times V = -g \nabla z_s + \frac{1}{h} \nabla \cdot (v_{\sharp} h \nabla V) - \frac{\tau_b}{\rho R} + \frac{\tau_s}{\rho h}$$

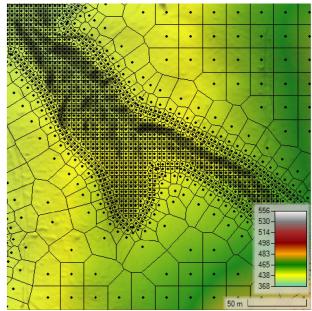
The modified governing equation's goal is to guarantee stabile and faster calculation. The time step for calculating overland flow is small, around or under 1 seconds.

Also an eddy viscosity model was added:  $\vartheta_t = Du^*h + (C_s\Delta)^2 |\bar{S}|$ The geometry was based on LiDAR, where adaptive mesh was created

For the unsaturated soil, same Green & Ampt model was used, with the same soil type distribution.

#### **Calibrated values:**

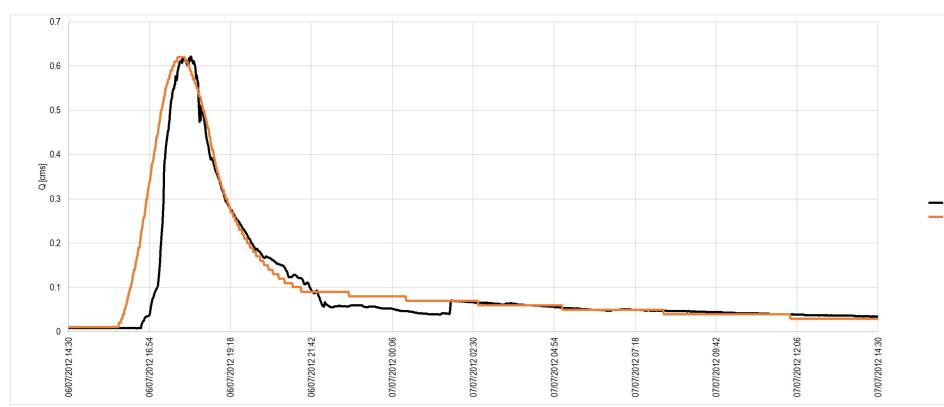
Manning's n, forest = 0.16 Manning's n, stream = 0.04 K [mm/h] = 26.667 Sf [mm] = 219.05 Initial content = 0.179 Saturated content = 0.353Resudial content = 0.0314 Pore size distribution = 0.395

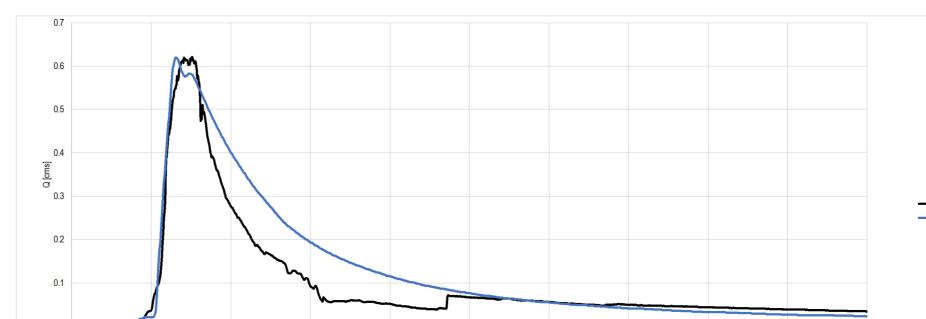


Part of generated mesh

## Results on the outlow section

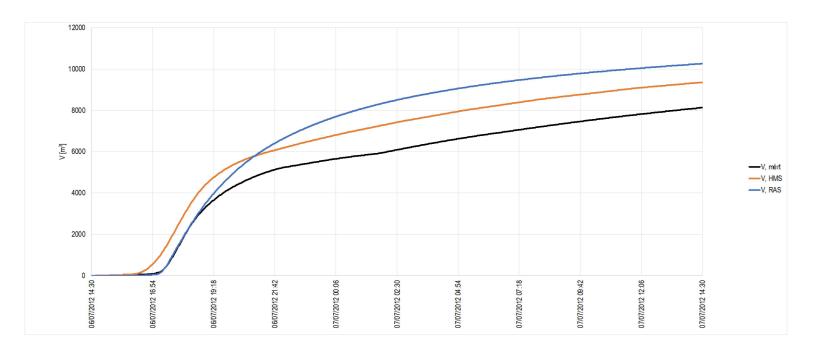
rResults





0											
e	54	18	42	90	30	54	18	42	900	30	
4	16	19	51	8	6	8	20	8	12	14	
012	012	012	012	2012	012	012	012	2012	2012	012	
5/20	2/20	2/20	5/20	2/20	2/20	2/20	2/20	2/20	2/20	2/20	
06/	06/	06/	06/	/20	/20	/20	/20	/20	//20	//20	

#### **Cumulated outflow volume**



## **Conclusion:**

- 1. Both models can be calibrated for peak flow, for time of concentration or for decreasing flow the models have errors. Therefore the outflow volume has differences as well, although the ratio of overland flow in both results are around 3% put the number in
- Models with different physical content on a same area need slightly different calibrations and some parameters' sensitivity can differ ez ide nem való
- 3. It is suggested that the models' with different physical content can be used as comparational studies on ungauged watersheds for flash-flood prediction ez nem ennek a konklúziója

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írd le amit látni a rajzon-rising limb with hms, recession limb with ras + a hec ras eredménye problémák, hec ras was aable to generate runoff hydrograph, but need more refinement-can be used for